Traffic Impact Study For New Office, Shop, and Plant for Kansas Sand and Concrete

at

NE Corner of NW 25th St. & NW Stina Ct.

Project Location:

Northeast corner of NW 25th St. & Stina Ct. Topeka, KS

Client:

Kansas Sand & Concrete Inc. 531 SW Tyler St. Topeka, KS 66608

Report Prepared by:



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SBB Project No. 20-174

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Introduction

Kansas Sand and Concrete, Inc. is developing 10.2 acres at the northeast corner of NW 25th St. and NW Stina Ct. The area being developed is zoned as I2 – Heavy Industrial District. This Traffic Impact Study (TIS) will estimate the traffic impact of this development on the adjacent road network and make recommendations for geometric improvements or changes in intersection control, if warranted.



Kansas Sand and Concrete Project Location

The study area of the traffic impact study includes the intersection of NW 25th & Stover Rd., NW 25th St. & NW Stina Ct., and the proposed accesses for the development along NW Stina Ct. and NW 25th St.



Kansas Sand and Concrete Preliminary Site Plan showing proposed access points.

Existing Conditions

The study area has the following characteristics -

NW 25th St. & Stover Rd. -

NW Stover Rd. is a four-lane divided roadway that was once a portion of US-75. The intersection at NW 25th St. & Stover Rd. is a T-intersection with the northbound approach being stop controlled. There are turn lanes for both the right turn and left turn movements. NW 25th St is a two-lane asphalt road and has uncontrolled eastbound and westbound approaches without turn lanes. NW Stover Rd. does not have a posted speed limit. NW 25th St. has a posted speed limit of 45 mph.

NW 25th St. & Stina Ct. –

NW Stina Ct. is a 2-lane road with a cul-de-sac approximately 1000-feet north of NW 25th St. NW Stina Ct. at 25th Street is stop controlled and does not have turn lanes while the eastbound and westbound approaches on NW 25th St. are uncontrolled. At the time of this study, NW Stina Ct. only served one industrial property, which is Hoyt Trailer Center. NW Stina Ct. does not have a posted speed limit.



NW 25th & Stover Rd. (looking west)

Existing Traffic Analysis

Morning and afternoon peak hour counts were performed at NW 25th & Stover Rd. and NW 25th & Stina Ct. on Tuesday, October 6. The AM count was performed between 7:00am-9:00am and the PM count was performed between 4:00pm-6:00pm. The AM peak hour occurred between 7:30am and 8:30am and the PM peak hour occurred between 4:00pm and 5:00pm. The traffic count data is provided in Appendix A.

The AM and PM peak hour data was entered into Synchro traffic modeling software to determine the existing intersection Level of Service. The results for the AM and PM peak hour are shown in the tables below. The Synchro results which are based on the Highway Capacity Manual (6th Edition) formulas show both intersections operating at Level of Service (LOS) A in both the AM and PM peak hours. All movements are LOS A and there are no queuing concerns at any of the approaches.

				←16 ▲15		Stina Ct. 1	► 0 ← 30
NW 2	25th S	t.	23 → 20 →	24 9 15 4		4 . ₩ 32→	
			AM Peak H	Hour Traffic	Volumes		
			Level of S NW 2 AM Peak I	Service (LOS) 5th & Stover Hour - Existir	Results Rd. g Traffic		
Approach	Approach Delay (s)	Approach LOS	Movement	Lane Group Delay (s)	Movement LOS	Average (50%) Queue (veh)	95% Queue (veh)
CD	0.0		Left	N/A	N/A	N/A	N/A
LD	0.0		Right	0.0	Δ	0	0
			Left	7.5	Δ	0	0
WB	3.6	Δ	Thru	0.0	Δ	0	0
	0.0		Right	N/A	N/A	N/A	N/A
			Left	9.4	Δ	0.1	0.1
NB	9.2	Δ	Thru	N/A	N/A	N/A	N/A
			Right	8.8	A	0.1	0.1
			Left	N/A	N/A	N/A	N/A
SB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
Inte	ersection De	elay	4.2		Intersection	LOS	A
			Level of S	Service (LOS)	Results		
			AM Dook I	Hour - Evistin	or Traffic		
	Approach	Approach	- Curr Curr	Lane Group	Movement	Average (50%)	95%
Approach	Delay (s)	LOS	Movement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	7.4	A	0	0
EB	0.8	Α	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	N/A	N/A	N/A	N/A
WB	0.0	Α	Thru	0.0	A	0	0
			Right	0.0	Α	0	0
			Left	N/A	N/A	N/A	N/A
NB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
			Left	8.6	А	0	0
SB	8.6	Α	Thru	N/A	N/A	N/A	N/A
			Right	0.0	А	0	0
Inte	ersection D	elay	0.6		Intersection	LOS	А



PM Peak Hour Traffic Volumes

			Level of 9 NW 2	Service (LOS) 5th & Stover	Results Rd.		
			PM Peak I	Hour - Existin	ig Traffic		
Approach	Approach	Approach	Movement	Lane Group	Movement	Average (50%)	95%
Approach	Delay (s)	LOS	wovement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	N/A	N/A	N/A	N/A
EB	0.0	Α	Thru	0.0	Α	0	0
			Right	0.0	Α	0	0
			Left	7.5	Α	0.1	0.1
WB	4.5	А	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	9.6	Α	0.1	0.1
NB	9.1	А	Thru	N/A	N/A	N/A	N/A
			Right	8.7	Α	0.1	0.1
			Left	N/A	N/A	N/A	N/A
SB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
Inte	ersection De	elay	5.1		Intersection	LOS	А

			Level of S	Service (LOS)	Results		
			NW	25th & Stina	Ct.		
			PM Peak I	Hour - Existin	g Traffic		
Ammenach	Approach	Approach	Maurant	Lane Group	Movement	Average (50%)	95%
Approach	Delay (s)	LOS	wovement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	7.4	А	0	0
EB	0.2	Α	Thru	0.0	А	0	0
			Right	N/A	N/A	N/A	N/A
			Left	N/A	N/A	N/A	N/A
WB	0.0	Α	Thru	0.0	А	0	0
			Right	0.0	А	0	0
			Left	N/A	N/A	N/A	N/A
NB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
			Left	8.8	А	0	0
SB	8.8	Α	Thru	N/A	N/A	N/A	N/A
			Right	0.0	А	0	0
Inte	ersection De	elay	0.7		Intersection	LOS	А

Proposed Development

The proposed development will be 10.2 acres of I2-Heavy Industrial Zoning. The proposed building will be a 22,900 square foot building of combined office, plant, and shop space for servicing and loading cement trucks.

There will be five new accesses for the plant. One access will be for ingress only for returning concrete truck traffic and raw material deliveries. This access will be spaced approximately 310 feet east of NW Stina Ct. on NW 25th St. This distance meets the minimum corner clearance distance for a minor arterial from the City of Topeka Design Criteria 1.2.5.1.2. There will be four access points on NW Stina Ct. The first driveway is approximately 240-feet north of NW 25th Street. This is greater than the 80-feet corner clearance required for a local road. The remaining driveway spacings are approximately 110-feet, 65-feet, and 140-feet. City of Topeka Design Criteria allows 1 driveway for the first 135-feet of frontage and additional driveways for each additional 300-feet of frontage. The proposed development has 1,120 feet of frontage, including the cul-de-sac, so the number of proposed driveways meets the City of Topeka Design Criteria 1.2.5.1.1 for non-residential driveways on a local street.

The 65-foot driveway spacing between the second and third driveways does not meet the minimum 80-foot spacing required by the City of Topeka Design Criteria. However, the 65-foot spacing for these driveways was designed to align with the truck service bays and material delivery vehicle paths to allow for better on-site traffic circulation. A variance is requested to allow the 65-foot driveway spacing as shown in the preliminary site plan. This variance request is based on NW Stina Ct. being a cul-de-sac local street serving only two low volume industrial developments.

Trip Generation

The Institute of Transportation Engineer's (ITE) Trip Generation Handbook, 10th Edition, was used to estimate the total number of trips generated by the proposed development on the street network. The development is zoned I2 – Heavy Industrial. The ITE Land Use Codes 110 'General Light Industrial' and 140 'Manufacturing' are similar land uses in the Trip Generation Handbook that were used to estimate the volume of traffic that will enter and leave the site.

			Тг	Trip Generation					
Gross Floor Area	22,900	F			Vehicle Tri	ps per Gross	s Floor Area	a	
Acres	10.2	Land Use	h	А	M Peak Ho	ur	Р	M Peak Ho	ur
		Code	Weekday	Total	Inbound	Outbound	Total	Inbound	Outbound
General Light Indu	strial	110	114	16	14	2	14	1	13
Manufacturing		140	90	14	11	3	15	4	11
Estimates from KS	Sand	N/A	242	39	29	10	39	10	29

These land uses and associated trip generation estimates were compared to the estimates provided by Kansas Sand & Concrete based on their past data. The user provided data includes:

Estimated Average Vehicle Trips

- 1) Ready Mix Concrete Trucks (Outgoing) 47 Loads @ average total weight of 60,000 lbs. (Max weight of 74,000)
- 2) Cement Truck/Trailers (Inbound) 7 loads @ average total weight of 80,000 lbs.
- 3) Aggregate End Dumps (inbound) 32 loads @ average total weight of 80,000 lbs.
- 4) 35 employee vehicles 70 daily trips

The result of the developer provided data of 172 trucks and 70 passenger car trips totals 242 daily trips for the site. This volume is higher than the estimated traffic from the ITE Trip Generation Handbook and therefore will be the volume used for Existing + Development traffic analysis scenario.

Trip Distribution

The AM and PM peak hour traffic was added to the existing traffic to determine the Existing + Development Traffic scenario. The distribution of the 39 peak hour generated vehicles was estimated using engineering judgment and through correspondence with the plant manager. The AM and PM peak hour distributions were estimated as shown in the figures below.



PM Peak Hour Trip Generation and Distribution

Existing + Development Traffic Analysis

The existing traffic was added to the site generated traffic to determine the Existing + Development traffic scenario. The resulting volumes and Synchro model analysis for the Existing + Development AM peak hour are shown in the figure and tables below.



Estimated Existing + Development AM Peak Hour Traffic Volumes

			Level of S	Service (LOS)	Results		
			NW 2	25th & Stover	Rd.		
		AM P	eak Hour - Ex	kisting + Deve	elopment Tra	affic	
Approach	Approach	Approach	Movement	Lane Group	Movement	Average (50%)	95%
Арргоасн	Delay (s)	LOS	wovement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	N/A	N/A	N/A	N/A
EB	0.0	Α	Thru	0.0	Α	0	0
			Right	0.0	Α	0	0
			Left	7.5	Α	0	0
WB	4.1	Α	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	9.6	Α	0.1	0.1
NB	9.2	Α	Thru	N/A	N/A	N/A	N/A
			Right	8.9	Α	0.1	0.1
			Left	N/A	N/A	N/A	N/A
SB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
Inte	ersection D	elay	4.7		Intersection	LOS	А

			Level of S	Service (LOS)	Results		
			NW	25th & Stina	Ct.		
		AM P	eak Hour - Ex	kisting + Deve	elopment Tra	affic	
Approach	Approach	Approach	Movement	Lane Group	Movement	Average (50%)	95%
Арргоасн	Delay (s)	LOS	wovement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	7.4	Α	0	0.1
EB	3.0	Α	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	N/A	N/A	N/A	N/A
WB	0.0	Α	Thru	0.0	Α	0	0
			Right	0.0	Α	0	0
			Left	N/A	N/A	N/A	N/A
NB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
			Left	8.8	Α	0	0
SB	8.8	Α	Thru	N/A	N/A	N/A	N/A
			Right	0.0	Α	0	0
Inte	ersection D	elay	2.7		Intersection	LOS	А

The resulting volumes and Synchro model analysis for the Existing + Development PM peak hour are shown in the figures below.



Estimated Existing + Development PM Peak Hour Traffic Volumes

			Level of 9 NW 2	Service (LOS) 5th & Stover	Results Rd.		
		PM P	eak Hour - Ex	cisting + Deve	elopment Tra	affic	
Approach	Approach	Approach	Movement	Lane Group	Movement	Average (50%)	95%
npproden	Delay (s)	LOS	morement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	N/A	N/A	N/A	N/A
EB	0.0	Α	Thru	0.0	Α	0	0
			Right	0.0	Α	0	0
			Left	7.5	Α	0.1	0.1
WB	4.8	Α	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	9.9	А	0.1	0.1
NB	9.2	Α	Thru	N/A	N/A	N/A	N/A
			Right	8.8	Α	0.1	0.1
			Left	N/A	N/A	N/A	N/A
SB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
Inte	ersection De	elay	5.2	I	Intersection	LOS	А
		PM P	Level of S NW eak Hour - Ex	Service (LOS) 25th & Stina visting + Deve	Results Ct. elopment Tra	affic	
	Approach	Approach		Lane Group	Movement	Average (50%)	95%
Approach	Delay (s)	LOS	Movement	Delay (s)	LOS	Queue (veh)	Queue (veh)
			Left	7.4	Α	0	0.1
EB	0.7	Α	Thru	0.0	Α	0	0
			Right	N/A	N/A	N/A	N/A
			Left	N/A	N/A	N/A	N/A
WB	0.0	Α	Thru	0.0	A	0	0
			Right	0.0	Α	0	0
			Left	N/A	N/A	N/A	N/A
NB	N/A	N/A	Thru	N/A	N/A	N/A	N/A
			Right	N/A	N/A	N/A	N/A
			Left	9.0	Α	0	0.1
SB	9.0	Α	Thru	N/A	N/A	N/A	N/A
			Right	9.0	A	0	0.1
Inte	ersection D	elay	2.5		Intersection	LOS	А

The traffic analysis for both the AM and PM peak hour show both intersections operating at LOS A. There are no capacity or queuing concerns at any of the approaches.

Turn Lane Analysis

The City of Topeka Design Criteria 1.2.5.2.8 provides warrants for Right and Left Turn Deceleration Lanes.

Right Turn Deceleration Lane – A right turn deceleration lanes shall be required if:

- 1) The street's ADT exceeds 10,000 vehicles per day,
- 2) The street's operating speeds equal or exceed 35 mph,
- 3) The driveway's volume equals or exceeds 1,000 vehicles per day, and
- 4) The driveway's right turn ingress movements equal or exceed 40 vehicles per hour.

At both the westbound approaches to the proposed driveway on NW 25th St. and NW Stina Ct., none of the above criteria is met and therefore a right turn lane is not required.

Left Turn Deceleration Lane – A left turn deceleration lane shall be required if:

- 1) The street's ADT exceeds 10,000 vehicles per day,
- 2) The street's operating speeds equal or exceeds 35 mph,
- The driveway's volume equals or exceeds 1,000 vehicles per day, and
- 4) The driveway's left turn ingress movements exceed 10 percent of the street's peak period traffic volume or 100 vehicles per hour.

At both the eastbound approaches to the proposed driveway on NW 25th St. and NW Stina Ct., none of the above criteria is met and therefore a left turn lane is not required.

Sight Distance Analysis

The minimum intersection sight distance at NW 25th & Stina Ct. is the stopping sight distance for 45 mph which is 360-feet. The intersection sight distance for both the east and westbound egress movement at NW Stina Ct. well exceeds 360-feet and therefore the minimum intersection sight distance is met. Any monument signs that may be constructed for the development should be placed outside of the intersection sight distance area/triangle at the site.

Summary and Recommendations

The proposed Kansas Sand and Concrete development at the northeast corner of NW 25th & Stina Ct. will add approximately 39 AM and PM peak hour vehicles to adjacent street network along NW 25th Street including the intersection at NW Stover Rd. The additional vehicles will not adversely impact the capacity or traffic operations at either of the two study area intersections. All approaches at both intersections operate at LOS A in existing conditions and will continue to operate at LOS A after the proposed development. Changes to intersection control or geometric improvements are not required.

The 65-foot driveway spacing between the second and third driveway on NW Stina Ct. does not meet the minimum 80-foot spacing required by the City of Topeka Design Criteria. However, the 65-foot spacing for these driveways was designed to align with the truck service bays and material delivery vehicle paths to allow for better on-site traffic circulation. A variance is requested to allow the 65-foot driveway spacing as shown in the preliminary site plan.

APPENDIX A

Traffic Data

















































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APPENDIX B

Synchro Reports

Int Delay, s/veh	4.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4			- द	۲.	1
Traffic Vol, veh/h	23	20	15	16	24	15
Future Vol, veh/h	23	20	15	16	24	15
Conflicting Peds, #/hr	0	13	13	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	14	14	14	14	14	14
Mvmt Flow	26	23	17	18	27	17

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0 62	0 103	51	
Stage 1	-		- 51	-	
Stage 2	-		- 52	-	
Critical Hdwy	-	- 4.24	- 6.54	6.34	
Critical Hdwy Stg 1	-		- 5.54	-	
Critical Hdwy Stg 2	-		- 5.54	-	
Follow-up Hdwy	-	- 2.326	- 3.626	3.426	
Pot Cap-1 Maneuver	-	- 1468	- 867	984	
Stage 1	-		- 942	-	
Stage 2	-		- 941	-	
Platoon blocked, %	-	-	-		
Mov Cap-1 Maneuver	· -	- 1450	- 846	972	
Mov Cap-2 Maneuver	-		- 846	-	
Stage 1	-		- 931	-	
Stage 2	-		- 930	-	
Approach	EB	WB	NB		

Арргоасн	LD	VVD	ND	
HCM Control Delay, s	0	3.6	9.2	
HCM LOS			А	

Minor Lane/Major Mvmt	NBLn11	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	846	972	-	-	1450	-
HCM Lane V/C Ratio	0.032	0.018	-	-	0.012	-
HCM Control Delay (s)	9.4	8.8	-	-	7.5	0
HCM Lane LOS	А	А	-	-	A	А
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0	-

Int Delay, s/veh	0.6							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		÷.	et 👘		Y			
Traffic Vol, veh/h	4	32	30	0	0	1		
Future Vol, veh/h	4	32	30	0	0	1		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	-	-	-	0	-		
Veh in Median Storage,	# -	0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	15	15	15	15	15	15		
Mvmt Flow	4	36	33	0	0	1		

Major/Minor	Major1	Ν	/lajor2		Minor2		
Conflicting Flow All	33	0	-	0	77	33	
Stage 1	-	-	-	-	33	-	
Stage 2	-	-	-	-	44	-	
Critical Hdwy	4.25	-	-	-	6.55	6.35	
Critical Hdwy Stg 1	-	-	-	-	5.55	-	
Critical Hdwy Stg 2	-	-	-	-	5.55	-	
Follow-up Hdwy	2.335	-	-	-	3.635	3.435	
Pot Cap-1 Maneuver	1499	-	-	-	895	1004	
Stage 1	-	-	-	-	957	-	
Stage 2	-	-	-	-	946	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	r 1499	-	-	-	892	1004	
Mov Cap-2 Maneuver	r -	-	-	-	892	-	
Stage 1	-	-	-	-	954	-	
Stage 2	-	-	-	-	946	-	
Approach	EB		WB		SB		
HCM Control Delay, s	s 0.8		0		8.6		
HCM LOS					А		
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR 3	SBLn1	
Capacity (veh/h)		1499	-	-	-	1004	
HCM Lane V/C Ratio		0.003	-	-	-	0.001	
HCM Control Delay (s	s)	7.4	0	-	-	8.6	
HCM Lane LOS		А	А	-	-	А	
HCM 95th %tile Q(ve	h)	0	-	-	-	0	

Int Delay, s/veh	5.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4			- सी	<u>۲</u>	1
Traffic Vol, veh/h	16	19	33	22	24	29
Future Vol, veh/h	16	19	33	22	24	29
Conflicting Peds, #/hr	0	13	13	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	7	7	7	7	7
Mvmt Flow	18	21	36	24	26	32

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0 52	0 138	42	
Stage 1	-		- 42	-	
Stage 2	-		- 96	-	
Critical Hdwy	-	- 4.17	- 6.47	6.27	
Critical Hdwy Stg 1	-		- 5.47	-	
Critical Hdwy Stg 2	-		- 5.47	-	
Follow-up Hdwy	-	- 2.263	- 3.563	3.363	
Pot Cap-1 Maneuver	-	- 1522	- 844	1015	
Stage 1	-		- 968	-	
Stage 2	-		- 915	-	
Platoon blocked, %	-	-	-		
Mov Cap-1 Maneuve	r -	- 1503	- 814	1002	
Mov Cap-2 Maneuve	r -		- 814	-	
Stage 1	-		- 956	-	
Stage 2	-		- 893	-	
Approach	EB	WB	NB		
HCM Control Delay,	s 0	4.5	9.1		
HCM LOS			А		

Minor Lane/Major Mvmt	NBLn11	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	814	1002	-	-	1503	-
HCM Lane V/C Ratio	0.032	0.032	-	-	0.024	-
HCM Control Delay (s)	9.6	8.7	-	-	7.5	0
HCM Lane LOS	А	А	-	-	Α	Α
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0.1	-

Int Delay, s/veh	0.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		÷.	4		Y		
Traffic Vol, veh/h	1	41	51	3	2	5	
Future Vol, veh/h	1	41	51	3	2	5	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	80	80	80	80	80	80	
Heavy Vehicles, %	6	6	6	6	6	6	
Mvmt Flow	1	51	64	4	3	6	

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	68	0	-	0	119	66
Stage 1	-	-	-	-	66	-
Stage 2	-	-	-	-	53	-
Critical Hdwy	4.16	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.254	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1508	-	-	-	867	987
Stage 1	-	-	-	-	947	-
Stage 2	-	-	-	-	959	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1508	-	-	-	866	987
Mov Cap-2 Maneuver	-	-	-	-	866	-
Stage 1	-	-	-	-	946	-
Stage 2	-	-	-	-	959	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		8.8	
HCM LOS					A	
Minor Lang/Major Mun	nt	EDI	EDT			DIn1
	ш	EDL	EDI	VVDI	WDR	
Capacity (veh/h)		1508	-	-	-	949
HCM Lane V/C Ratio	、	0.001	-	-	-	0.009
HCM Control Delay (s)	1.4	0	-	-	8.8
HCM Lane LOS		A	A	-	-	A
HCM 95th %tile Q(veh	1)	0	-	-	-	0

Int Delay, s/veh	4.7						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ef 👘			- द	۳	1	
Traffic Vol, veh/h	28	20	21	18	24	30	
Future Vol, veh/h	28	20	21	18	24	30	
Conflicting Peds, #/hr	0	13	13	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	88	88	88	88	88	88	
Heavy Vehicles, %	14	14	14	14	14	14	
Mvmt Flow	32	23	24	20	27	34	

Major/Minor	Major1	Major2	Minor1	
Conflicting Flow All	0	0 68	0 125	57
Stage 1	-		- 57	-
Stage 2	-		- 68	-
Critical Hdwy	-	- 4.24	- 6.54	6.34
Critical Hdwy Stg 1	-		- 5.54	-
Critical Hdwy Stg 2	-		- 5.54	-
Follow-up Hdwy	-	- 2.326	- 3.626	3.426
Pot Cap-1 Maneuver	-	- 1460	- 842	976
Stage 1	-		- 936	-
Stage 2	-		- 925	-
Platoon blocked, %	-	-	-	
Mov Cap-1 Maneuve	r -	- 1442	- 818	964
Mov Cap-2 Maneuve	r -		- 818	-
Stage 1	-		- 925	-
Stage 2	-		- 909	-
Approach	EB	WB	NB	

Approach	FR	WB	NB	
HCM Control Delay, s	0	4.1	9.2	
HCM LOS			А	

Minor Lane/Major Mvmt	NBLn11	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	818	964	-	-	1442	-
HCM Lane V/C Ratio	0.033	0.035	-	-	0.017	-
HCM Control Delay (s)	9.6	8.9	-	-	7.5	0
HCM Lane LOS	А	А	-	-	А	А
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0.1	-

Int Delay, s/veh	2.7									
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		÷	et -		Y					
Traffic Vol, veh/h	24	36	30	1	2	9				
Future Vol, veh/h	24	36	30	1	2	9				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Free	Free	Free	Free	Stop	Stop				
RT Channelized	-	None	-	None	-	None				
Storage Length	-	-	-	-	0	-				
Veh in Median Storage,	# -	0	0	-	0	-				
Grade, %	-	0	0	-	0	-				
Peak Hour Factor	90	90	90	90	90	90				
Heavy Vehicles, %	15	15	15	15	15	15				
Mvmt Flow	27	40	33	1	2	10				

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	34	0	-	0	128	34
Stage 1	-	-	-	-	34	-
Stage 2	-	-	-	-	94	-
Critical Hdwy	4.25	-	-	-	6.55	6.35
Critical Hdwy Stg 1	-	-	-	-	5.55	-
Critical Hdwy Stg 2	-	-	-	-	5.55	-
Follow-up Hdwy	2.335	-	-	-	3.635	3.435
Pot Cap-1 Maneuver	1497	-	-	-	836	1003
Stage 1	-	-	-	-	956	-
Stage 2	-	-	-	-	898	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1497	-	-	-	821	1003
Mov Cap-2 Maneuver	· -	-	-	-	821	-
Stage 1	-	-	-	-	939	-
Stage 2	-	-	-	-	898	-
Approach	EB		WB		SB	
HCM Control Delay, s	3		0		8.8	
HCM LOS					А	
Minor Lane/Major Mvr	mt	EBL	EBT	WBT	WBR 3	SBLn1
Capacity (veh/h)		1497	-	_	-	964
HCM Lane V/C Ratio		0.018	-	-	-	0.013
HCM Control Delay (s	5)	7.4	0	-	-	8.8
HCM Lane LOS		А	А	-	-	А
HCM 95th %tile Q(vel	h)	0.1	-	-	-	0

Int Delay, s/veh	5.2						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ef 👘			- द	۳	1	
Traffic Vol, veh/h	19	19	49	28	24	35	
Future Vol, veh/h	19	19	49	28	24	35	
Conflicting Peds, #/hr	0	13	13	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage	,# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	91	91	91	91	91	91	
Heavy Vehicles, %	7	7	7	7	7	7	
Mvmt Flow	21	21	54	31	26	38	

Major/Minor	Major1	Major2	Minor1	
Conflicting Flow All	0	0 55	0 184	45
Stage 1	-		- 45	-
Stage 2	-		- 139	-
Critical Hdwy	-	- 4.17	- 6.47	6.27
Critical Hdwy Stg 1	-		- 5.47	-
Critical Hdwy Stg 2	-		- 5.47	-
Follow-up Hdwy	-	- 2.263	- 3.563	3.363
Pot Cap-1 Maneuver	-	- 1519	- 794	1011
Stage 1	-		- 965	-
Stage 2	-		- 876	-
Platoon blocked, %	-	-	-	
Mov Cap-1 Maneuver	r -	- 1500	- 755	998
Mov Cap-2 Maneuver	r -		- 755	-
Stage 1	-		- 953	-
Stage 2	-		- 844	-
Approach	EB	WB	NB	
HCM Control Delay, s	5 0	4.8	9.2	

HCM LOS	A

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	755	998	-	-	1500	-	
HCM Lane V/C Ratio	0.035	0.039	-	-	0.036	-	
HCM Control Delay (s)	9.9	8.8	-	-	7.5	0	
HCM Lane LOS	А	А	-	-	Α	А	
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0.1	-	

Int Delay, s/veh	2.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ب	et 👘		Y		
Traffic Vol, veh/h	5	45	51	4	9	27	
Future Vol, veh/h	5	45	51	4	9	27	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	80	80	80	80	80	80	
Heavy Vehicles, %	6	6	6	6	6	6	
Mvmt Flow	6	56	64	5	11	34	

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	69	0	-	0	135	67
Stage 1	-	-	-	-	67	-
Stage 2	-	-	-	-	68	-
Critical Hdwy	4.16	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.254	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1507	-	-	-	849	985
Stage 1	-	-	-	-	946	-
Stage 2	-	-	-	-	945	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1507	-	-	-	846	985
Mov Cap-2 Maneuver		-	-	-	846	-
Stage 1	-	-	-	-	942	-
Stage 2	-	-	-	-	945	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.7		0		9	
HCM LOS					А	
Minor Lane/Major Mvi	mt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1507	-	-	-	946
HCM Lane V/C Ratio		0.004	-	-	-	0.048
HCM Control Delay (s	5)	7.4	0	-	-	9
HCM Lane LOS		А	А	-	-	А
HCM 95th %tile Q(vel	h)	0	-	-	-	0.1